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S.E. (Civil) (Second Semester) EXAMINATION, 2010

SURVEYING

(2008 COURSE)

Time: Three Hours

Maximum Marks: 100

- N.B. :- (i) Answer three questions from Section I and three questions from Section II.
- (ii) Answers to the two Sections should be written in separate answer-books.
 - (iii) Neat diagrams must be drawn wherever necessary.
 - (iv) Figures to the right indicate full marks.
 - (v) Use of logarithmic tables, slide rule, Mollier charts, electronic pocket calculator and steam tables is allowed.
 - (vi) Assume suitable data, if necessary.

(b) regain to orief intersection method of plane table survey. When SECTION I

- (a) What is meant by orientation in plane table survey? Explain 1. method of backsighting for orientation. [5]
- (b) A closed traverse was run along a square PQRS in counterclockwise direction. The bearing of the line PQ was observed as 118° 30'. Find the Forebearings and Backbearings of the remaining lines and record them in a tabular form.

(c) Differentiate between Dip and Declination. The magnetic bearing of the sun at noon is 356° 30′. Find out the declination.

Or

2. (a) While carrying out compass traverse, the following Forebearings and Backbearings of various lines were observed. Correct the bearings affected by local attraction and enter your result in tabular form:
[7]

Line	Observed FB	Observed BB
PQ	110° 00′	292° 00′
and QR	40° 00′	220° 00′
RS	320° 30′	140° 00′
ST	270° 00′	91° 00′

- (b) Explain in brief intersection method of plane table survey. When is it used? [5]
- (c) Write in tabular form, how you will convert the bearings from Reduced Bearing system to WCB system. [6]
- 3. (a) What is meant by interpolation of contours? State various methods of it and explain any one in detail. [6]

(b) Reciprocal levels were taken with dumpy level as under: [6]

Instrument	Reading on		Remarks	
at	P	Q		
P	1.500	2.250	Distance between	
.Q	0.600	1.320	P and Q = 880 m	

Assuming collimation error to be -0.005 in 100 m. Calculate the true difference between P and Q and the correction for curvature, refraction and collimation.

(c) Write a short note on profile levelling.

[4]

Or

- 4. (a) Differentiate between collimation plane method and Rise and Fall method. [4]
 - (b) State and define various fundamental axes of dumpy level.

 Also write the relationships between them. [6]
 - (c) Derive an expression for the combined correction for curvature and refraction effect in levelling. [6]
- of one side and bearing of other side is omitted? [6]

(b) Define the following terms:

[4]

- (1) Vertical Asix
- (2) Trunion Axis
- (3) Optical Axis
- (4) Line of Collimation.
- traverse ABCD. Compute the missing length and whole circle bearing of side DA of the traverse: [6]

Line	Latitude	Departure
AB	-116.10	-44.40
BC	+6.80	+58.20
CD	+80.50	+17.20
DA	?	?

Or

- 6. (a) What do you mean by prolongation of a straight line? How is it done using 20" Theodolite? [6]
 - (b) Define deflection angle. Explain in brief the procedure of measurement of the deflection angle using 20" vernier transit theodolite. [6]

Define the following terms with neat sketch: [4] (c) (1) Closing error of traverse Independent co-ordinates (2)(3) Open traverse (4) Closed traverse. SECTION II (a) State the functions of the following parts of a Theodolite: [6] Clip screw (1) Optical plumet Evepiece (3)Shifting head (4) Bubble tube (5)Lower tangent screw. (6) A staff was held vertically at a distance of 125 m and (b) 50 m from the centre of a Tacheometer. The staff intercepts with the telescope horizontal were 1.248 and 0.498 respectively. Calculate the constants of a tacheometer. [6] Explain the necessary test and adjustment for making the (c) vertical circle to read zero when the line of collimation is

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[6]

horizontal.

- 8. (a) Describe in detail the field procedure of determining the constants of a Tacheometer. [6]
 - (b) Write a short note on Tacheometric Contour Survey. [4]
 - (c) A tacheometer was set up at an intermediate point between two stations A and B and the following observations were made on a vertically held staff:

 [8]

Staff Station	Vertical Angle	Staff Readings	
A	+4° 30′	1.605, 2.400, 3.195	
В	-2° 45′	0.805, 1.345, 1.885	

The instrument is fitted with an anallatic lens having a constant of 100. Compute the length AB and R.L. of point B, if that of A was 395.400 m. The instrument and staff stations are in one straight line.

- 9. (a) Draw the neat sketches of the following: [4]
 - (1) Simple circular curve
 - (2) Compound curve
 - (3) Reverse curve
 - (4) Valley curve.
 - (b) Work out the relationships between the elements of a simple circular curve. [4]

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	(c)	Two straights of road intersects at a chainage of 2550.50 m.
		The angle of intersection being 110°. Taking chord length of
		30 m, calculate the following: [8]
		(1) Radius of curve
		(2) Length of curve
		(3) Tangent length
		(4) Length of long chord
		(5) Chainages at the starting point and end point.
		or Or
10.	(a)	Write a short note on necessity of vertical curves in
		highways. [4]
	(b)	Describe in detail the method of setting out a simple circular
		curve by offset from chord produced. [6]
	(c)	A transition curve is to be designed for the following
		data: [6]
		(1) Radius of circular curve = 300 m
		(2) Gauge = 1.5 m
		(3) Maximum superelevation = 15 cm
		(4) No lateral pressure on rails
		(5) Rate of gain of radial acceleration = 0.3 m/sec^3 .
		Find the length of the curve and design speed.

11.	(a)	State the classification of EDM instruments.	[5]
	(b)	Explain in brief the process of setting out a building	on
		ground.	[5]
	(c)	What is total station? State any five special functions availa	ble
		in total station.	[6]
		Sometiment of the second of th	
		Or	
12.	(a)	Define gradient. What is the importance of gradient while lay	ing
		sewer pipe ? How is it decided ?	[6]
	(b)	Write a short note on basic principle of EDM Instruments.	[5]
	(c)	Write a short note on different types of construct	ion
		survey.	[5]
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		ave by offset have chord produced.	

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