	B.E.Civil (Semester-I	
	Structural Design-III	
	(2008 Course)	
Time: 04 H	rs Max.Marks: 100	
1] Answer 2] Answers 3] Figures 4] IS 456-2 5] Assume	s to the candidates : Q-1 or Q-2, Q-3 or Q-4, from section-I And Q-5 or Q-6 Q-7 or Q-8 from Section-II to the two sections should be written in separate books to the right indicates full marks 000, IS 1343, IS 3370 are allowed suitable data wherever necessary lectronic non programmable calculator is allowed	
0 1 / 4 \	SECTION-I	[[]]
Que-1 (A)	Explain Systems of Prestressing	[5]
(B) (C)	Explain need of High grade materials in prestressing systems An unsymmetrical I-section is used to support an imposed load of 15 KN/m over a span of 15 m, the sectional details are top flange 600 x200 mm, web 200 x 700 mm, and bottom flange 350x350 mm, the effective prestressing force of 950 KN is located 100mm from soffit of the section at mid span, cable profile is parabolic concentric at support, calculate extreme fiber stresses in concrete at mid span at initial and final stage, take loss ratio as 0.85 and unit weight of concrete as 25 KN/m ³	[5]
	OR	
Que-2(A)	Explain in detail losses of Prestress due to friction	[5]
(B)	State advantages and disadvantages of Prestress Concrete	[5]
(C)	A post tensioned prestress concrete beam section has top flange 400x150mm, web 125 x 500 mm and bottom flange 350 x 200 mm, is simply supported over a span of 12 m. the beam is prestressed with 3 no's of 12/7 Freyssinet parabolic cables with fy =1500 Mpa with their C.G at 120 mm from extreme bottom fiber, stressed one at a time from both the ends, Calculate the total loss of prestress at the age of 100 days, if the coefficient of friction =0.28, k=0.0026/m length of cable, Slip of anchorage is 1.5 mm, Cc=2.1, Es=210 Gpa, Concrete grade M45, Creep and relaxation of steel =2% of initial stress	[15]
Que-3	Design a post tensioned prestressed concrete T or 1 – Section beam for flexure to carry a live load of 15 KN/m over a simply supported span of 18 m with M 45 grade of concrete and Freyssinet cables of 12/5 (f_{y} = 1750 Mpa) or 12/7(f_{y} = 1500 Mpa), including the design of end block, draw sketches showing cable profile and end block reinforcement details, check for stresses in concrete & deflection is required	[25]
000 4/41	OR State what are the remedial measures to be taken to reduce losses in prestress?	[5]
Que 4(A) (B)	A Post tensioned prestressed concrete two way slab $6mx 9m$, with discontinuous edge to support the imposed load of 3.5 KN/m^2 , Use S3 strands each having area of 100 mm^2 and F_{y} = 1900 Mpa , check the safety of slab against collapse and deflection at service loads use M40 Concrete	[20]

	SECTION-II	
Que-5	Fig(1) shows an intermediate frame of multistoried building, the frames are spaced at 5m centre to centre, analyze the rigid jointed frame taking Live load and Dead load of Slab 3 KN/m ² and 4 KN/m ² for panels AB and BC respectively, the self weight of beam AB is 4.5 KN/m and for BC is 3.00 KN/m, the relative stiffness are marked on the figure, use portal method for analyzing the frame for horizontal forces and proper substitute frame for vertical loads. Design the section ABC for combined effect of vertical and horizontal loads. Adopt 15 % redistribution of moments for vertical load moments Use M20 and Fe 500	[25]
	$15 \text{ KN} \longrightarrow 7m$ 5m	
	4K 3K	
	^{3m} K K K	
	$10 \text{ KN} \rightarrow A 4 \text{K} \text{ B} 3 \text{K} \text{ C}$	
	3m 2K 2K 2K	
	$5 \text{ KN} \rightarrow 4 \text{K} - 3 \text{K}$	
	3.5m 3K 3K	
	31.	
	mhn mhn mhn	
	OR	
Que-6(A)	Write detail note on Substitute frame method	[8]
(B)	Analyze the rigid jointed frame shown in fig (2) by cantilever method for lateral loads , flexural rigidity of all members is same , analyze the beam DEF using	[17]
	proper substitute frame if it is subjected to vertical ultimate Live and Dead load	
	including its self weight of intensities 18 KN/m and 20 KN/m on DE and 22 KN/m	
	and 25 KN/m on EF respectively , calculate maximum span moment for DE and	
	support moment at E , Design the beam DEF for combined effect of vertical and Horizontal loads adopt 15 % redistribution of moments for vertical load moments	
	Use M20 and Fe 500	
	$A \longrightarrow B \qquad C, \qquad 5KN$	
	3.5m	
	3.5m E F	
	$G \xrightarrow{4m} H 6m \xrightarrow{6m} I5KN$	
	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	

Que-7	Design a T-shape retaining wall for leveled backfill along with surcharge, the height of wall is 5 met. Φ =30 ⁰ , γ =18 KN/m ³ , and additional surcharge at the face of wall is 6 KN/m ² , The coefficient of friction between the base slab and under laying strata is 0.52, Use M25 and Fe500, draw the lateral pressure diagram and details of reinforcement of stem and base slab showing curtailment if any	[25]
Series Colors	OR	
Que-8(A)	Compare slab type and slab and beam type of combined footing	[5]
(B)	 Design a rectangular combined footing without rib for the following data 1) C/C Distance between the columns=3.5m 2) Working Load on First column=400KN 3) Working Load on Second column=500KN 4) S.B.C of soil=175 KN/m² 5) Material =M20 and Fe=415 Draw SFD and BMD , Show all detail calculations , show complete details of reinforcement 	[20]