

Q.1. (a) stress-strain curve for concrete - (02)
HYSD steel - (02) } = (04)

(b) Each durability aspect = $0.1 \times 6 =$ 06

Q. 2. (a) Each parameter = $01 \times 4 = 04$

(b) Each failure mode = $(0.2) \times 3 = 0.6$

Q.3. (a)

(i) $A_{st} = 603 \text{ mm}^2$

$$\sigma_{cbc} = 7 \text{ N/mm}^2$$

$$\sigma_{st} = 230 \text{ N/mm}^2$$

$$m = 280/36 \text{ cbc}$$

$$= 13 \cdot 33.$$

(2) critical neutral axis (n_c)

$$\frac{m_{\text{Sebe}}}{\Delta t} = \frac{n_c}{d - n_c}$$

$$n_c = 144.13 \text{ mm}$$

(3) Actual neutral axis (n)

$$\frac{bn^2}{2} = m \Delta s t (d-n)$$

$n = 139.1 \text{ mm}$

(4) As $n_c > n$, The section

is under reinforced.

(5) Moment of resistance

$$M_{or} = \sigma_{st} \cdot A_{st} \cdot (d - \frac{n}{3})$$

$$= 62.91 \text{ kNm}$$

$$(6) \text{ BM} = \frac{Wl^2}{8} = 62.91$$

$$w = 31.455 \text{ rad/s}$$

$$\omega = \omega_d + \omega_i$$

$$31.455 = 0.3 \times 0.5 \times 25 + w_i$$

$$\therefore W_i = 27.705 \text{ kN}$$

Q. 3. (b) (i) Actual NA (n)

Actual NA (1)
 $A_{st} = 4 \times 491 = 1964 \text{ mm}^2$

$$\frac{b\eta^2}{2} = m \cdot A \cdot t \cdot (d - \eta)$$

$$n = 222.32 \text{ mm.}$$

(2) Moment of resistance

$$M_r = \sigma_{st} A_{st} \left(d - \frac{n}{3}\right) = \sigma_{st} \times 1964 \left(600 - \frac{222.32}{3}\right) \\ = 1031931.804 \sigma_{st} \text{ Nmm.}$$

(3) Stress in steel (σ_{st})

$$M = M_r.$$

$$150 \times 10^6 = 1031931.804 (\sigma_{st})$$

$$\therefore \sigma_{st} = 145.36 \text{ N/mm}^2$$

(4) Stress in concrete (σ_c)

$$\frac{\sigma_c}{n} = \frac{\sigma_{st}/m}{d-n}$$

$$\sigma_c = \frac{145.36 \times 222.32}{13.33 (600 - 222.32)}$$

$$\therefore \sigma_c = 6.42 \text{ N/mm}^2$$

Q. 4.(1) $b = 350 \text{ mm}$

$$d = 700 - 50 = 650 \text{ mm}$$

$$\sigma_{cbc} = 7 \text{ N/mm}^2$$

$$\sigma_{st} = 140 \text{ N/mm}^2$$

$$m = \frac{280}{3 \times 7} = 13.33$$

(2) Design constants.

$$K = \frac{m \sigma_{cbc}}{m \sigma_{cbc} + \sigma_{st}} = 0.29, \quad j = 1 - \frac{K}{3} = 0.90$$

$$R = \frac{1}{2} \sigma_{cbc} K j = 0.91$$

$$n = K d = 188.5 \text{ mm}$$

(3) MR of balanced section = M_1

$$M_1 = R b d^2 = 134.566 \text{ kNm.}$$

As $M > M_1$, design section as doubly reinforced

$$M_2 = M - M_1 = 50.433 \text{ kNm}$$

(4) A_{st} - tension steel

$$A_{st1} = \frac{M_1}{\sigma_{st} j d} = 1000.12 \text{ mm}^2$$

$$A_{st2} = \frac{M_2}{\sigma_{st} (d - d')} = 365.5 \text{ mm}^2$$

$$A_{st} = A_{st1} + A_{st2} = 1365.62 \text{ mm}^2$$

(5) Compression steel (A_{sc})

$$A_{sc} = \frac{m A_{st2} (d - n)}{(1.5m - 1) (n - d')}$$

$$= 197.3 \text{ mm}^2$$